

# DESIGN REPORT

Bituminous pavement - 130.9 msa

Designed in accordance with IRC:37-2018 (mechanistic-empirical method)

|                           |                                                                                                          |
|---------------------------|----------------------------------------------------------------------------------------------------------|
| <b>Project</b>            | -                                                                                                        |
| <b>Client</b>             | -                                                                                                        |
| <b>Location</b>           | -                                                                                                        |
| <b>Chainage / section</b> | -                                                                                                        |
| <b>Design</b>             | IRC 37 example check                                                                                     |
| <b>Designed by</b>        | -                                                                                                        |
| <b>Checked by</b>         | -                                                                                                        |
| <b>Design traffic</b>     | 130.91 msa (90 % reliability)                                                                            |
| <b>Pavement (mm)</b>      | Bituminous layers 100 + Aggregate interlayer 100 + Cement treated base 180 + Cement treated sub-base 250 |
| <b>Result</b>             | ALL CHECKS SATISFIED                                                                                     |
| <b>Date</b>               | 31 Oct 2026                                                                                              |

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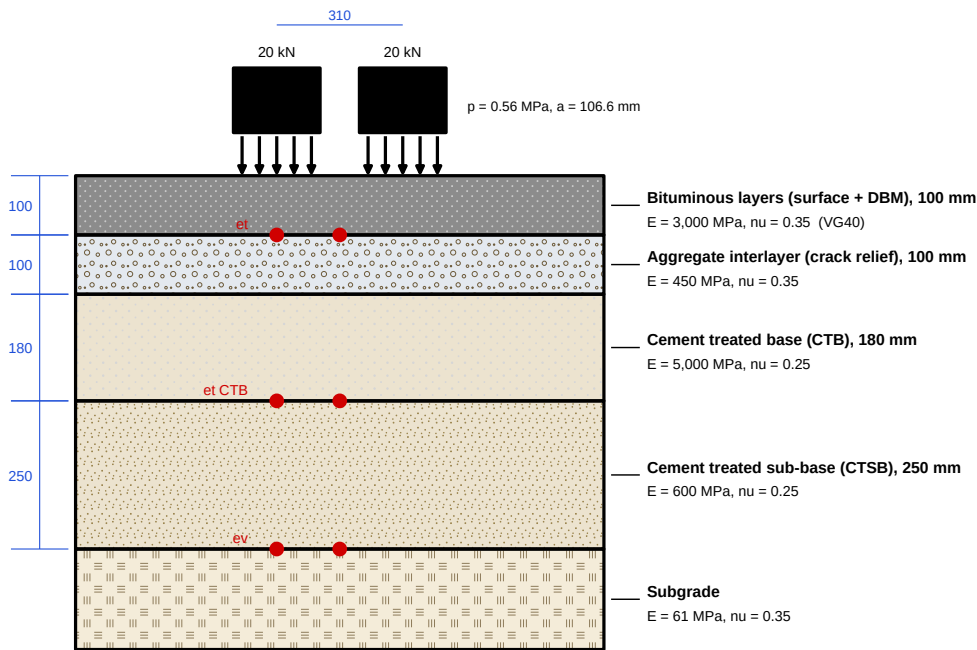
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1. Design basis and input data

Bituminous pavement designed to IRC:37-2018 (Guidelines for the Design of Flexible Pavements, fourth revision) by the mechanistic-empirical method: the pavement is analysed as a linear elastic, fully bonded layered system under the standard axle (dual wheels of 20 kN at 0.56 MPa, 310 mm c/c; 0.80 MPa for the cement treated base) and the computed critical strains are compared with the allowable strains of the performance models for subgrade rutting (Eq. 3.1/3.2), bottom-up fatigue cracking of the bituminous layer (Eq. 3.3/3.4) and fatigue of the CTB (Eq. 3.5 - 3.7). The layered elastic analysis performed by IITPAVE is carried out by this program (Burmister multilayer solution); its results agree with the IITPAVE values of Annexes II and III within about 1 %.

|                         |                                                                            |
|-------------------------|----------------------------------------------------------------------------|
| Project                 | -                                                                          |
| Client                  | -                                                                          |
| Location / chainage     | -                                                                          |
| Road category           | National Highway                                                           |
| Carriageway             | Dual carriageway - two lanes each way                                      |
| Composition             | Bituminous + aggregate interlayer + CTB + CTSB                             |
| Design period           | 20 years                                                                   |
| Bituminous binder (DBM) | BC and DBM with VG40 bitumen; average annual pavement temperature 35 deg C |

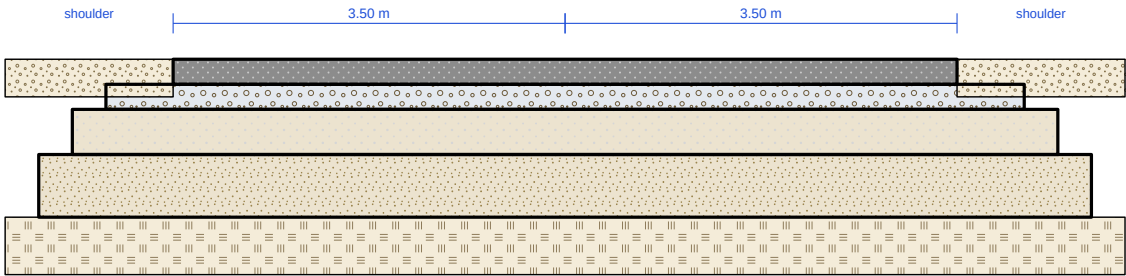
PAVEMENT COMPOSITION AND CRITICAL LOCATIONS



Thicknesses to scale vertically; subgrade semi-infinite. Red dots: critical strain locations (Table 3.1).

Fig. 1 Pavement composition, standard load and critical locations

TYPICAL CROSS SECTION (SCHEMATIC)



Dual carriageway - two lanes each way - layer thicknesses exaggerated; total pavement 630 mm.

Fig. 2 Typical cross section (schematic)

## 2. Design traffic

| Cumulative standard axles (Cl. 4.6) |                                                          |                                                                |                          | OK |
|-------------------------------------|----------------------------------------------------------|----------------------------------------------------------------|--------------------------|----|
| Reference                           | Description                                              | Calculation                                                    | Result                   |    |
| Cl. 4.5                             | Lateral distribution factor D                            | Dual carriageway - two lanes each way                          | 0.75                     |    |
| Eq. 4.6                             | Traffic in the year of completion $A = P (1 + r)^x$      | $5,000 \times 50 \% \text{ (directional)} \times (1 + 0.06)^0$ | 2,500 CVPD               |    |
| Cl. 4.4 / Table 4.2                 | Vehicle damage factor F                                  | axle load survey                                               | 5.2                      |    |
| Eq. 4.5                             | $N = 365 [(1 + r)^n - 1] / r \times A \times D \times F$ | $365 \times 36.786 \times 2,500 \times 0.75 \times 5.2$        | 130,910,723 = 130.91 msa |    |
| Cl. 3.7                             | Reliability of the performance models                    | important road                                                 | 90 %                     |    |

CUMULATIVE DESIGN TRAFFIC

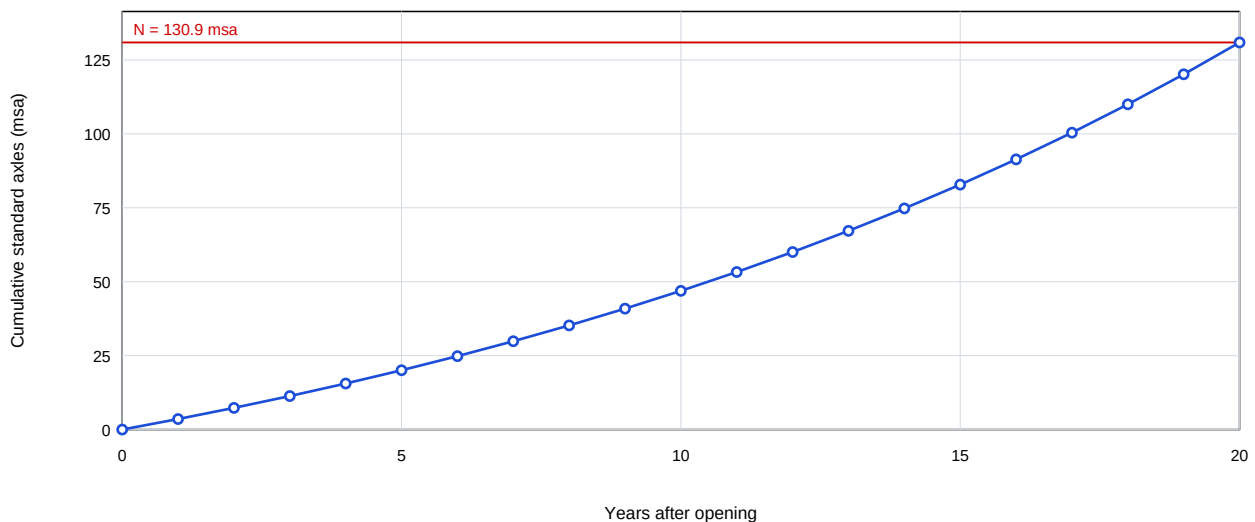


Fig. 3 Growth of cumulative standard axles over the design period

## 3. Subgrade - effective resilient modulus

| Resilient modulus of the subgrade (Cl. 6.3, 6.4) |                                          |                        |          | OK |
|--------------------------------------------------|------------------------------------------|------------------------|----------|----|
| Reference                                        | Description                              | Calculation            | Result   |    |
| Eq. 6.1 / 6.2                                    | Resilient modulus of the 500 mm subgrade | $17.6 \times 7^{0.64}$ | 61.1 MPa |    |
| Eq. 6.2                                          | Effective CBR reported                   |                        | 7.0 %    |    |

## 4. Layer properties for analysis

Layers analysed (Table 11.1); bituminous layers as one layer with the DBM modulus

| Layer                               | Thickness (mm) | E (MPa) | Poisson's ratio | Basis                                                |
|-------------------------------------|----------------|---------|-----------------|------------------------------------------------------|
| Bituminous layers (surface + DBM)   | 100            | 3,000   | 0.35            | Table 9.2 (BC and DBM with VG40 bitumen at 35 deg C) |
| Aggregate interlayer (crack relief) | 100            | 450     | 0.35            | Cl. 8.3 / Table 11.1                                 |
| Cement treated base (CTB)           | 180            | 5,000   | 0.25            | Cl. 8.2 / Table 11.1                                 |
| Cement treated sub-base (CTSB)      | 250            | 600     | 0.25            | Cl. 7.3.2 / Table 11.1                               |
| Subgrade                            | semi-infinite  | 61      | 0.35            | Cl. 6                                                |

| Bituminous mix parameters |                                                           |             |                        | OK |
|---------------------------|-----------------------------------------------------------|-------------|------------------------|----|
| Reference                 | Description                                               | Calculation | Result                 |    |
| Cl. 9.2                   | Volume of air voids / effective bitumen of the bottom DBM | mix design  | Va = 3 %, Vbe = 11.5 % |    |

| Reference | Description                                                       | Calculation                               | Result |  |
|-----------|-------------------------------------------------------------------|-------------------------------------------|--------|--|
| Eq. 3.3   | Mix adjustment C = $10^{(4.84 (V_{be} / (V_a + V_{be}) - 0.69))}$ | $10^{(4.84 \times (11.5 / 14.5 - 0.69))}$ | 3.155  |  |

## 5. Structural analysis - linear elastic layered system

Standard axle: one set of dual wheels, 20 kN each, 310 mm c/c, uniform contact stress 0.56 MPa (contact radius 106.6 mm); fully bonded interfaces (Table 3.1). Responses at the bottom of the bituminous layer, the bottom of the CTB and the top of the subgrade, under the centre of one wheel ( $x = 0$ ) and midway between the wheels ( $x = 155$  mm). Strains in microstrain, tension positive;  $x$  along the line joining the wheels.

### Computed responses at the critical locations

| Location                               | x (mm) | ex    | ey    | ez     | Vertical stress (MPa) |
|----------------------------------------|--------|-------|-------|--------|-----------------------|
| Bottom of bituminous layer, z = 100 mm | 0      | 89.7  | 127.5 | -172.6 | -0.2678               |
| Bottom of bituminous layer, z = 100 mm | 155    | -79.0 | 108.5 | -54.1  | -0.1840               |
| Bottom of CTB, z = 380 mm (0.80 MPa)   | 0      | 29.3  | 41.3  | -29.7  | sx 0.199 / sy 0.247   |
| Bottom of CTB, z = 380 mm (0.80 MPa)   | 155    | 33.0  | 44.3  | -32.7  | sx 0.222 / sy 0.266   |
| Top of subgrade, z = 630 mm            | 0      | 52.8  | 62.7  | -159.1 | -0.0095               |
| Top of subgrade, z = 630 mm            | 155    | 58.7  | 65.3  | -168.6 | -0.0100               |

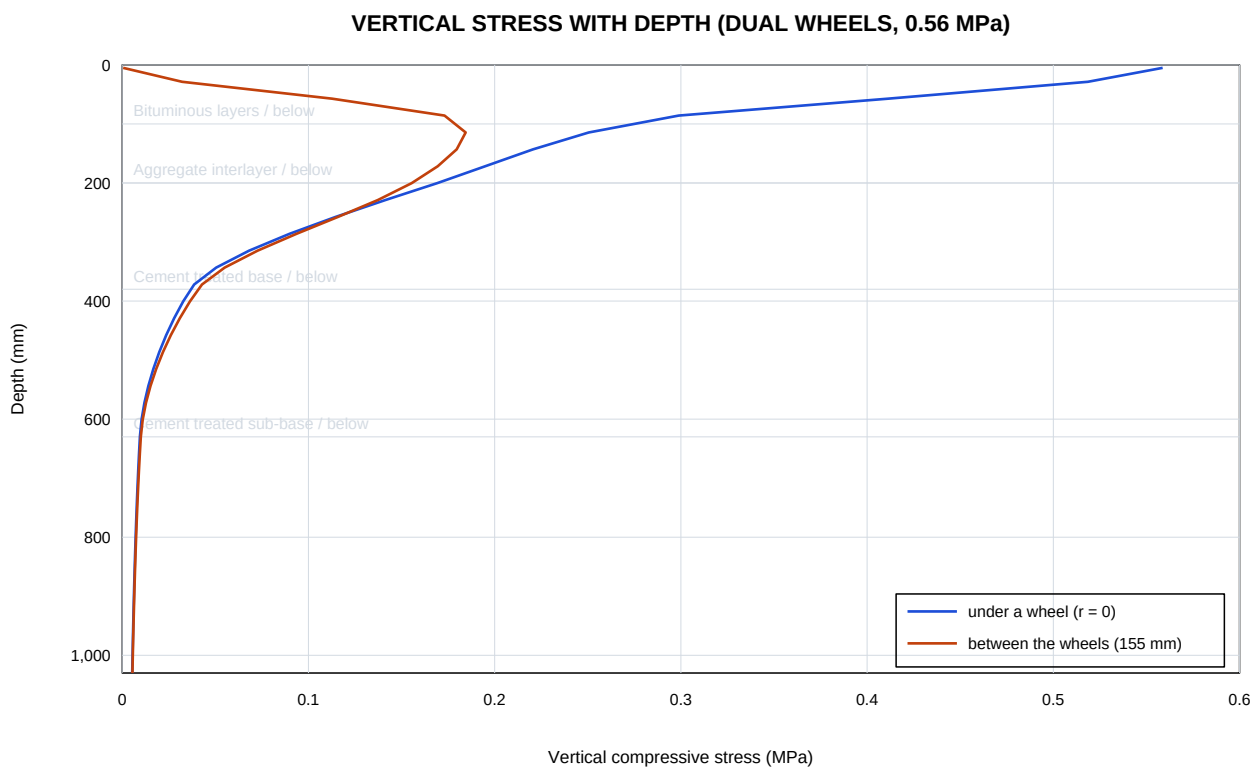


Fig. 4 Vertical compressive stress with depth

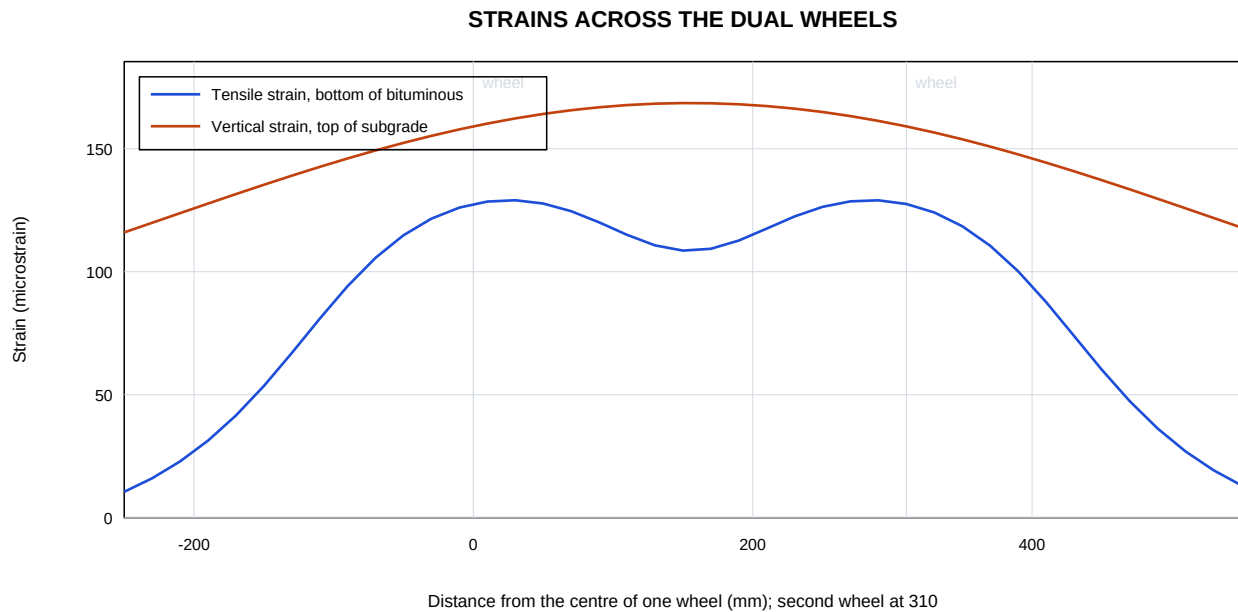


Fig. 5 Critical strains across the dual wheel set

## 6. Performance checks

| Performance criteria (Cl. 3.6) |                                                                   |                                                                                    |                   | OK |
|--------------------------------|-------------------------------------------------------------------|------------------------------------------------------------------------------------|-------------------|----|
| Reference                      | Description                                                       | Calculation                                                                        | Result            |    |
| Eq. 3.2                        | Allowable subgrade strain (N = design traffic)                    | $(1.4100e-8 / 130,910,723)^{(1/4.5337)}$                                           | 300.6 microstrain |    |
| Analysis                       | Computed vertical strain, top of subgrade                         | max. of x = 0 and 155 mm                                                           | 168.6 microstrain | OK |
| Eq. 3.2                        | Rutting life NR                                                   | $1.4100e-8 (1 / 1.686e-04)^{4.5337}$                                               | 1,802.90 msa      | OK |
| Eq. 3.4                        | Allowable tensile strain, bottom of bituminous layer              | $(0.5161 \times 3.155 \times 1e-4 \times 3,000^{-0.854} / 130,910,723)^{(1/3.89)}$ | 150.0 microstrain |    |
| Analysis                       | Computed tensile strain, bottom of bituminous layer               | max. of x = 0 and 155 mm                                                           | 127.5 microstrain | OK |
| Eq. 3.4                        | Fatigue life Nf                                                   | $0.5161 \times 3.155 \times 1e-4 (1 / 1.275e-04)^{3.89} (1 / 3,000)^{0.854}$       | 246.26 msa        | OK |
| Eq. 3.5                        | CTB life N = RF $[(113000 / E^{0.804} + 191) / et]^{12}$ , RF = 1 | $1 \times [(113000 / 5,000^{0.804} + 191) / 44.3]^{12}$                            | 14,400.79 msa     | OK |

### Summary of checks

| Check                                             | Reference     | Computed            | Limit               | Result |
|---------------------------------------------------|---------------|---------------------|---------------------|--------|
| Subgrade rutting - vertical strain on subgrade    | Eq. 3.1/3.2   | 168.561 microstrain | 300.598 microstrain | OK     |
| Bituminous fatigue - tensile strain at bottom     | Eq. 3.3/3.4   | 127.535 microstrain | 150.029 microstrain | OK     |
| CTB fatigue - life from tensile strain (0.80 MPa) | Eq. 3.5       | 130.911 msa         | 14,400.792 msa      | OK     |
| CTB under construction traffic (7-day strength)   | Eq. 3.6, II.4 | 0.698 MPa           | 0.795 MPa           | OK     |
| CTB cumulative fatigue damage (axle spectrum)     | Eq. 3.6/3.7   | 0.788               | 1.000               | OK     |
| Minimum layer thicknesses                         | Cl. 7 - 9     | 0 issue(s)          | 0                   | OK     |

## 7. Cumulative fatigue damage of the cement treated base

Tensile stress at the bottom of the CTB for every axle load class at 0.80 MPa contact stress; tandem axles as two and tridem axles as three single axles (Cl. 3.6.3.2); modulus of rupture 1.4 MPa;  $\log_{10} N_f = (0.972 - SR) / 0.0825$ .

### Single axles - CFD 0.089

| Load class (kN) | Axle load (kN) | Single axle reps ni | Stress (MPa) | SR    | Fatigue life Nf | ni / Nf |
|-----------------|----------------|---------------------|--------------|-------|-----------------|---------|
| 185 - 195       | 190            | 70,000              | 0.604        | 0.431 | 3.571e+06       | 0.0196  |
| 175 - 185       | 180            | 90,000              | 0.575        | 0.410 | 6.415e+06       | 0.0140  |

| Load class (kN) | Axle load (kN) | Single axle reps ni | Stress (MPa) | SR    | Fatigue life Nf | ni / Nf |
|-----------------|----------------|---------------------|--------------|-------|-----------------|---------|
| 165 - 175       | 170            | 92,000              | 0.545        | 0.389 | 1.158e+07       | 0.0079  |
| 155 - 165       | 160            | 300,000             | 0.515        | 0.368 | 2.102e+07       | 0.0143  |
| 145 - 155       | 150            | 280,000             | 0.485        | 0.346 | 3.833e+07       | 0.0073  |
| 135 - 145       | 140            | 650,000             | 0.454        | 0.325 | 7.027e+07       | 0.0092  |
| 125 - 135       | 130            | 600,000             | 0.424        | 0.303 | 1.295e+08       | 0.0046  |
| 115 - 125       | 120            | 1,340,000           | 0.393        | 0.281 | 2.399e+08       | 0.0056  |
| 105 - 115       | 110            | 1,300,000           | 0.362        | 0.258 | 4.469e+08       | 0.0029  |
| 95 - 105        | 100            | 1,500,000           | 0.330        | 0.236 | 8.369e+08       | 0.0018  |
| 85 - 95         | 90             | 1,350,000           | 0.298        | 0.213 | 1.576e+09       | 0.0009  |
| 75 - 85         | 80             | 3,700,000           | 0.266        | 0.190 | 2.984e+09       | 0.0012  |

**Tandem axles - CFD 0.548**

| Load class (kN) | Axle load (kN) | Single axle reps ni | Stress (MPa) | SR    | Fatigue life Nf | ni / Nf |
|-----------------|----------------|---------------------|--------------|-------|-----------------|---------|
| 390 - 410       | 400            | 400,000             | 0.633        | 0.452 | 1.998e+06       | 0.2002  |
| 370 - 390       | 380            | 460,000             | 0.604        | 0.431 | 3.571e+06       | 0.1288  |
| 350 - 370       | 360            | 480,000             | 0.575        | 0.410 | 6.415e+06       | 0.0748  |
| 330 - 350       | 340            | 470,000             | 0.545        | 0.389 | 1.158e+07       | 0.0406  |
| 310 - 330       | 320            | 450,000             | 0.515        | 0.368 | 2.102e+07       | 0.0214  |
| 290 - 310       | 300            | 950,000             | 0.485        | 0.346 | 3.833e+07       | 0.0248  |
| 270 - 290       | 280            | 900,000             | 0.454        | 0.325 | 7.027e+07       | 0.0128  |
| 250 - 270       | 260            | 2,870,000           | 0.424        | 0.303 | 1.295e+08       | 0.0222  |
| 230 - 250       | 240            | 2,500,000           | 0.393        | 0.281 | 2.399e+08       | 0.0104  |
| 210 - 230       | 220            | 2,370,000           | 0.362        | 0.258 | 4.469e+08       | 0.0053  |
| 190 - 210       | 200            | 2,000,000           | 0.330        | 0.236 | 8.369e+08       | 0.0024  |
| 170 - 190       | 180            | 1,600,000           | 0.298        | 0.213 | 1.576e+09       | 0.0010  |
| 150 - 190       | 170            | 6,400,000           | 0.283        | 0.202 | 2.167e+09       | 0.0030  |

**Tridem axles - CFD 0.151**

| Load class (kN) | Axle load (kN) | Single axle reps ni | Stress (MPa) | SR    | Fatigue life Nf | ni / Nf |
|-----------------|----------------|---------------------|--------------|-------|-----------------|---------|
| 585 - 615       | 600            | 105,000             | 0.633        | 0.452 | 1.998e+06       | 0.0526  |
| 555 - 585       | 570            | 120,000             | 0.604        | 0.431 | 3.571e+06       | 0.0336  |
| 525 - 555       | 540            | 120,000             | 0.575        | 0.410 | 6.415e+06       | 0.0187  |
| 495 - 525       | 510            | 135,000             | 0.545        | 0.389 | 1.158e+07       | 0.0117  |
| 465 - 495       | 480            | 129,000             | 0.515        | 0.368 | 2.102e+07       | 0.0061  |
| 435 - 465       | 450            | 330,000             | 0.485        | 0.346 | 3.833e+07       | 0.0086  |
| 405 - 435       | 420            | 300,000             | 0.454        | 0.325 | 7.027e+07       | 0.0043  |
| 375 - 405       | 390            | 990,000             | 0.424        | 0.303 | 1.295e+08       | 0.0076  |
| 345 - 375       | 360            | 900,000             | 0.393        | 0.281 | 2.399e+08       | 0.0038  |
| 315 - 345       | 330            | 825,000             | 0.362        | 0.258 | 4.469e+08       | 0.0018  |
| 285 - 315       | 300            | 780,000             | 0.330        | 0.236 | 8.369e+08       | 0.0009  |
| 255 - 285       | 270            | 540,000             | 0.298        | 0.213 | 1.576e+09       | 0.0003  |
| 240 - 270       | 255            | 2,160,000           | 0.283        | 0.202 | 2.167e+09       | 0.0010  |

Total CTB cumulative fatigue damage = 0.089 + 0.548 + 0.151 = 0.788 ≤ 1.0

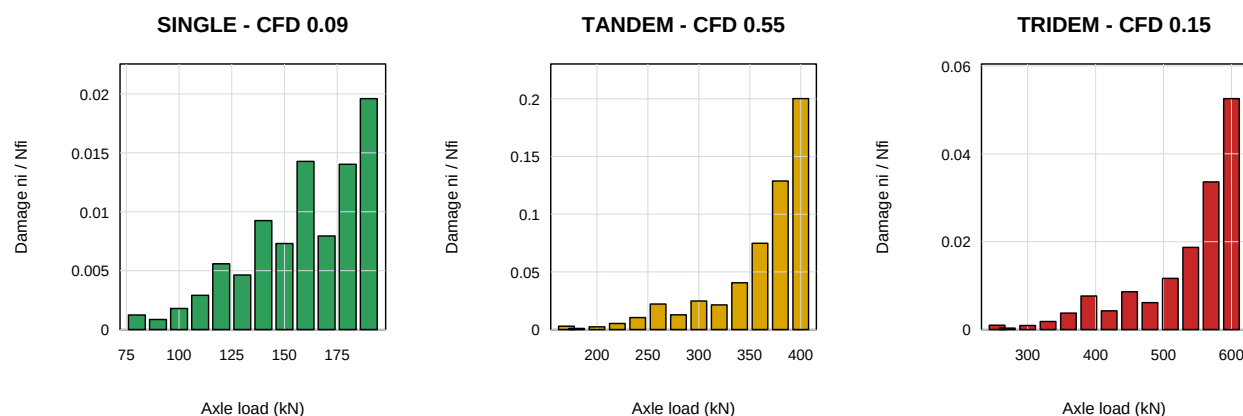


Fig. 6 CTB fatigue damage by axle load class

## 8. Construction traffic

| Sub-base / CTB under construction traffic (Cl. 7.2.2, Annex II) |                                                                       |                                       |                  | OK |
|-----------------------------------------------------------------|-----------------------------------------------------------------------|---------------------------------------|------------------|----|
| Reference                                                       | Description                                                           | Calculation                           | Result           |    |
| Annex II.2                                                      | VDF of a dumper (rear tandem as two single axles)                     | $2 \times (120 / 80)^4 + (80 / 65)^4$ | 12.42            |    |
| Cl. 7.2.2 (v)                                                   | Construction traffic (min. 10,000 SA)                                 | $\max(200 \times 12.42, 10000)$       | 10,000 SA        |    |
| Eq. 3.2                                                         | Allowable subgrade strain                                             |                                       | 2433 microstrain |    |
| Eq. 3.6                                                         | Allowable stress, 140 reps of 120 kN, 7-day MR 1 MPa                  | $1 (0.972 - 0.0825 \log 140)$         | 0.795 MPa        |    |
| Annex II.4                                                      | Tensile stress at bottom of CTB (CTB + sub-base + subgrade, 0.80 MPa) |                                       | 0.698 MPa        | OK |

## 9. Bituminous layer thickness trials

Other layers as given; total bituminous thickness varied. Least thickness satisfying all life checks: 100 mm.

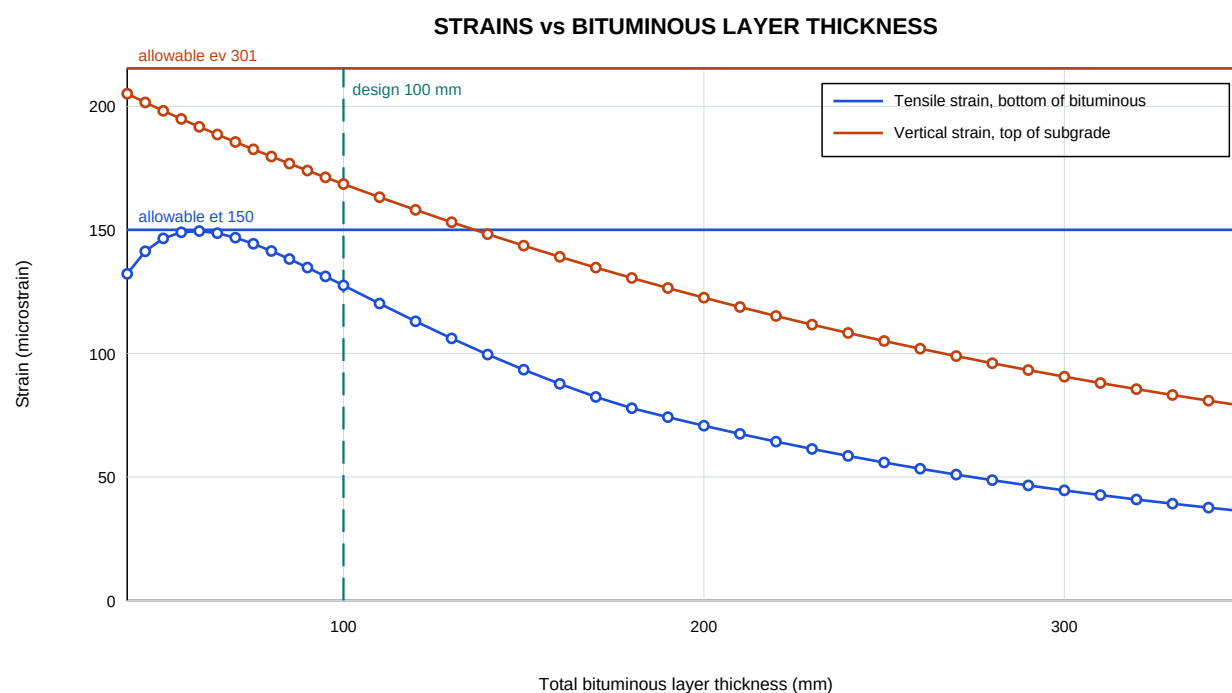


Fig. 7 Critical strains against bituminous layer thickness

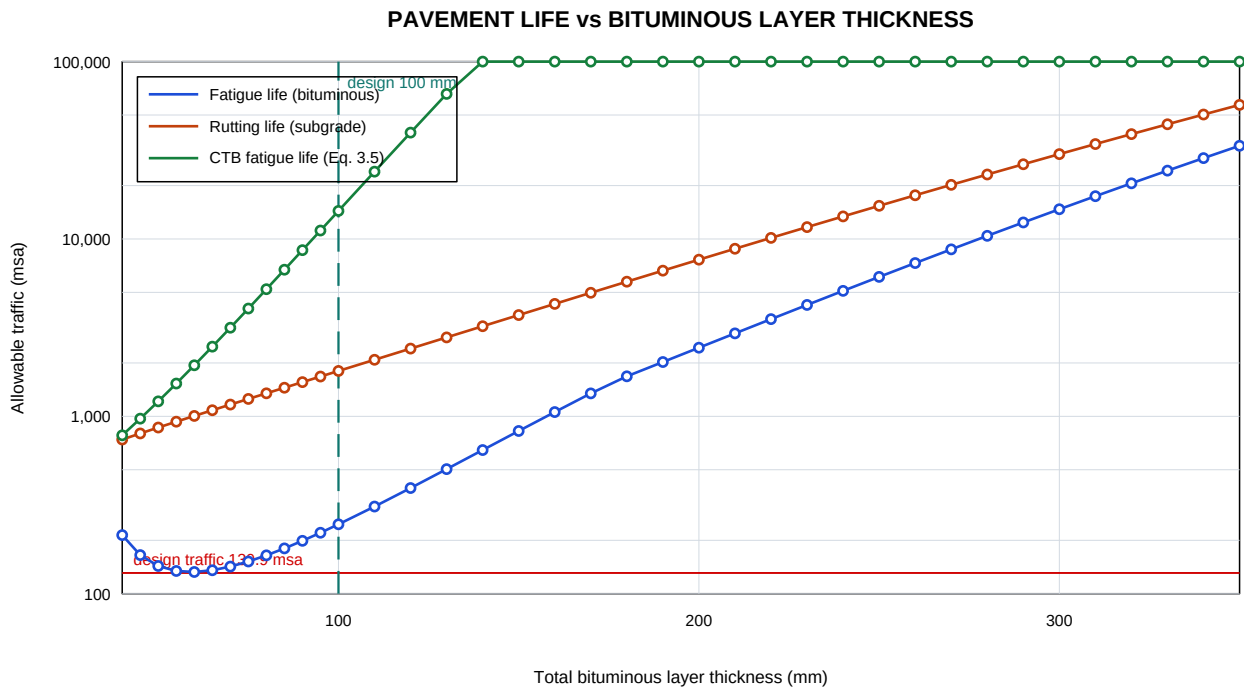


**Fig. 8 Allowable traffic against bituminous layer thickness**

**Trial thicknesses**

| Bituminous (mm) | et    | ev    | Fatigue life  | Rutting life  | CTB life           | Result |
|-----------------|-------|-------|---------------|---------------|--------------------|--------|
| 40              | 132.2 | 205.1 | 214.02 msa    | 740.17 msa    | 781.65 msa         | NOT OK |
| 45              | 141.3 | 201.6 | 165.12 msa    | 800.86 msa    | 970.69 msa         | NOT OK |
| 50              | 146.6 | 198.2 | 143.34 msa    | 865.01 msa    | 1,214.91 msa       | NOT OK |
| 55              | 149.0 | 194.9 | 134.36 msa    | 933.06 msa    | 1,531.00 msa       | NOT OK |
| 60              | 149.5 | 191.7 | 132.60 msa    | 1,005.47 msa  | 1,940.56 msa       | NOT OK |
| 65              | 148.7 | 188.6 | 135.61 msa    | 1,082.72 msa  | 2,471.57 msa       | NOT OK |
| 70              | 146.9 | 185.6 | 142.23 msa    | 1,165.30 msa  | 3,160.19 msa       | NOT OK |
| 75              | 144.4 | 182.6 | 151.95 msa    | 1,253.71 msa  | 4,053.11 msa       | NOT OK |
| 80              | 141.5 | 179.7 | 164.57 msa    | 1,348.48 msa  | 5,210.51 msa       | NOT OK |
| 85              | 138.2 | 176.9 | 180.11 msa    | 1,450.18 msa  | 6,709.82 msa       | NOT OK |
| 90              | 134.8 | 174.0 | 198.73 msa    | 1,559.40 msa  | 8,650.46 msa       | NOT OK |
| 95              | 131.2 | 171.3 | 220.67 msa    | 1,676.75 msa  | 11,159.82 msa      | NOT OK |
| 100             | 127.5 | 168.6 | 246.26 msa    | 1,802.90 msa  | 14,400.79 msa      | OK     |
| 110             | 120.2 | 163.3 | 310.10 msa    | 2,084.42 msa  | 23,965.69 msa      | OK     |
| 120             | 113.0 | 158.1 | 394.30 msa    | 2,410.05 msa  | 39,781.45 msa      | OK     |
| 130             | 106.1 | 153.1 | 504.05 msa    | 2,786.67 msa  | 65,759.59 msa      | OK     |
| 140             | 99.5  | 148.3 | 645.72 msa    | 3,222.06 msa  | 108,125.03 msa     | OK     |
| 150             | 93.4  | 143.6 | 827.05 msa    | 3,724.98 msa  | 176,697.59 msa     | OK     |
| 160             | 87.7  | 139.1 | 1,057.25 msa  | 4,305.32 msa  | 286,833.53 msa     | OK     |
| 170             | 82.4  | 134.8 | 1,347.22 msa  | 4,974.21 msa  | 462,341.05 msa     | OK     |
| 180             | 77.8  | 130.5 | 1,681.51 msa  | 5,744.19 msa  | 739,820.98 msa     | OK     |
| 190             | 74.2  | 126.5 | 2,024.38 msa  | 6,629.33 msa  | 1,175,075.16 msa   | OK     |
| 200             | 70.7  | 122.6 | 2,438.16 msa  | 7,645.42 msa  | 1,852,490.71 msa   | OK     |
| 210             | 67.4  | 118.8 | 2,935.99 msa  | 8,810.11 msa  | 2,898,674.29 msa   | OK     |
| 220             | 64.3  | 115.2 | 3,533.20 msa  | 10,143.14 msa | 4,502,109.69 msa   | OK     |
| 230             | 61.3  | 111.7 | 4,247.62 msa  | 11,666.52 msa | 6,941,290.36 msa   | OK     |
| 240             | 58.5  | 108.3 | 5,099.87 msa  | 13,404.77 msa | 10,624,692.23 msa  | OK     |
| 250             | 55.9  | 105.0 | 6,113.75 msa  | 15,385.12 msa | 16,147,175.89 msa  | OK     |
| 260             | 53.3  | 101.9 | 7,316.61 msa  | 17,637.81 msa | 24,369,035.83 msa  | OK     |
| 270             | 51.0  | 98.9  | 8,739.78 msa  | 20,196.35 msa | 36,526,069.18 msa  | OK     |
| 280             | 48.7  | 96.0  | 10,419.04 msa | 23,097.79 msa | 54,381,869.42 msa  | OK     |
| 290             | 46.6  | 93.3  | 12,395.15 msa | 26,383.08 msa | 80,437,256.95 msa  | OK     |
| 300             | 44.6  | 90.6  | 14,714.33 msa | 30,097.35 msa | 118,216,578.82 msa | OK     |

| Bituminous (mm) | et   | ev   | Fatigue life  | Rutting life  | CTB life           | Result |
|-----------------|------|------|---------------|---------------|--------------------|--------|
| 310             | 42.7 | 88.0 | 17,428.91 msa | 34,290.34 msa | 172,656,848.00 msa | OK     |
| 320             | 40.9 | 85.6 | 20,597.96 msa | 39,016.72 msa | 250,633,724.67 msa | OK     |
| 330             | 39.2 | 83.2 | 24,287.94 msa | 44,336.55 msa | 361,668,633.18 msa | OK     |
| 340             | 37.6 | 80.9 | 28,573.46 msa | 50,315.67 msa | 518,874,432.25 msa | OK     |
| 350             | 36.1 | 78.7 | 33,538.02 msa | 57,026.24 msa | 740,213,717.23 msa | OK     |

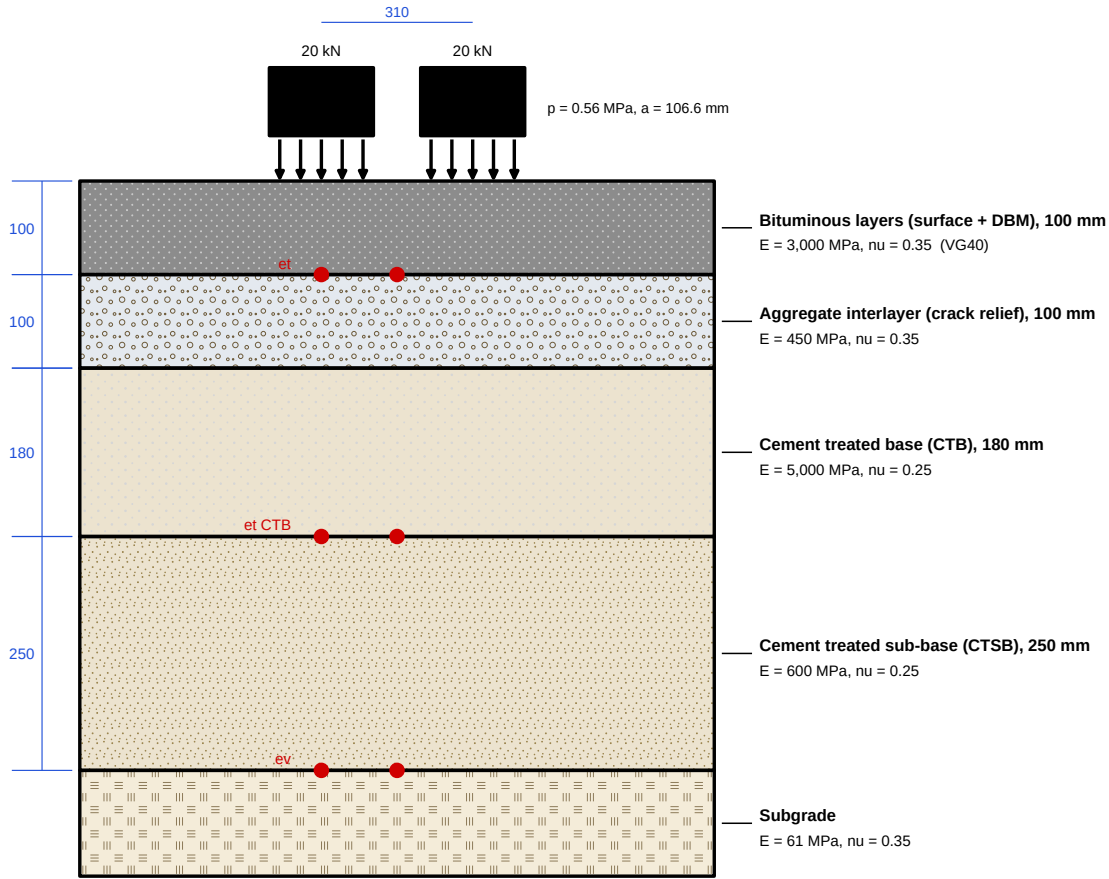
## 10. Design summary and recommendations

All checks are satisfied for the pavement composition: Bituminous layers 100 mm + Aggregate interlayer 100 mm + Cement treated base 180 mm + Cement treated sub-base 250 mm.

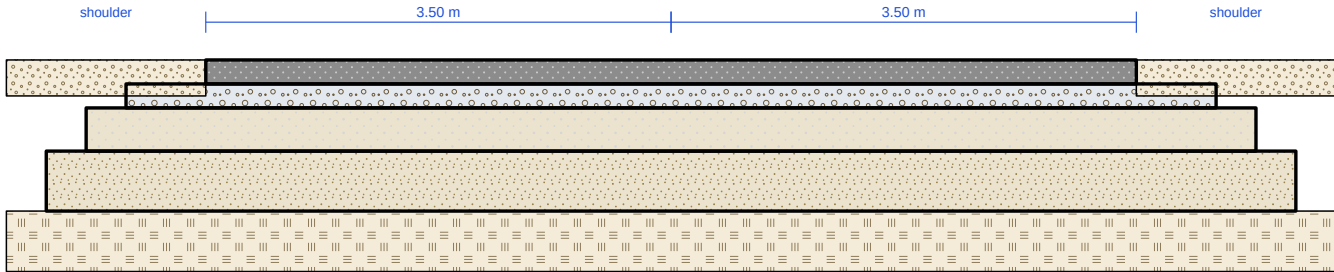
- Surface course: SMA (modified bitumen or VG40), GGRB (crumb rubber modified bitumen) or BC with modified bitumen (Table 9.1).
- Binder / base course: DBM with VG40 bitumen (viscosity  $\geq 3600$  Poise at 60 deg C).
- Bottom DBM: rich bottom mix designed for  $V_a = 3\%$  and  $V_{be} = 11.5\%$  (compacted to 4 % or lower in-place air voids for a two-layer DBM, 4.5 % for a single layer).
- Granular sub-base with drainage and filter layers extended over the full formation width (Cl. 7.2.1).
- Subgrade: top 500 mm compacted to 97 % of MDD (heavy compaction), effective CBR 7.0 %.

Report generated 31 Oct 2026.

PAVEMENT COMPOSITION AND CRITICAL LOCATIONS



TYPICAL CROSS SECTION (SCHEMATIC)

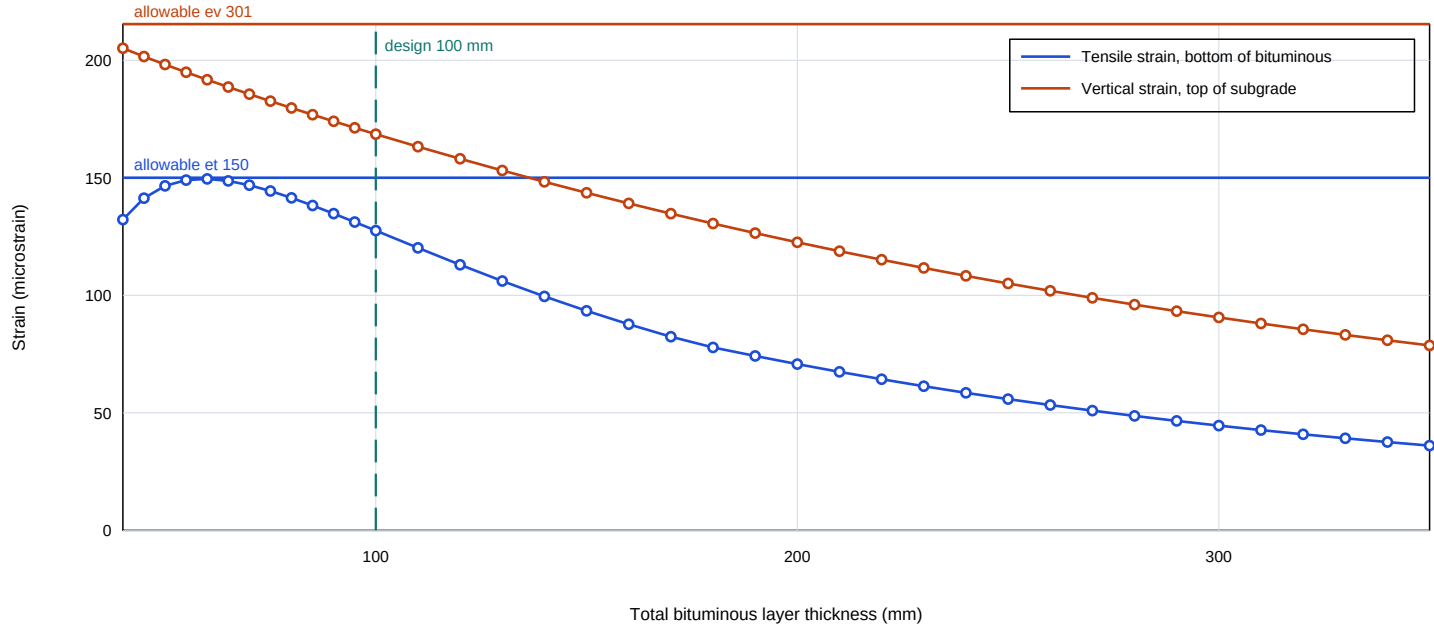


Dual carriageway - two lanes each way - layer thicknesses exaggerated; total pavement 630 mm.

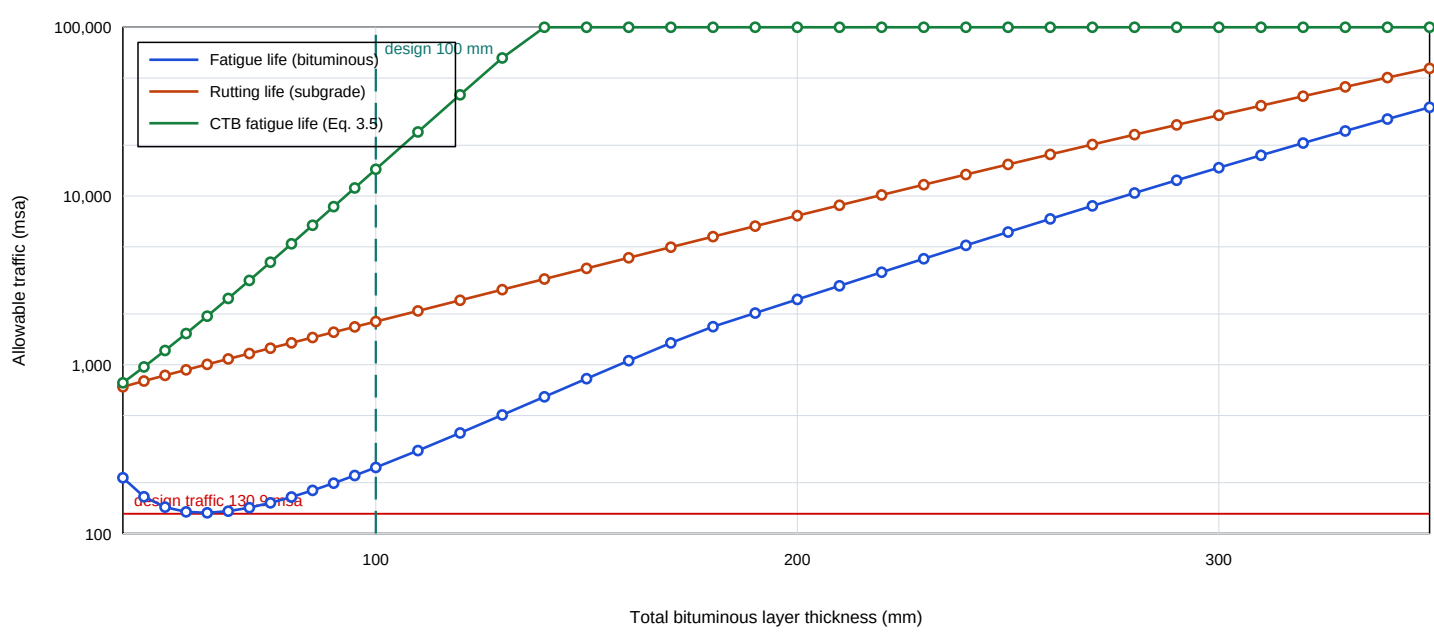
DESIGN SUMMARY (IRC:37-2018)

|                                                    |                                                                        |
|----------------------------------------------------|------------------------------------------------------------------------|
| Design traffic                                     | 130.9 msa (90 % reliability)                                           |
| Effective subgrade modulus                         | 61.1 MPa (CBR 7.0 %)                                                   |
| Composition                                        | Bituminous + aggregate interlayer + CTB + CTSB                         |
| Layers (mm)                                        | Bituminous layers 100 + Aggregate interlayer 100 + Cement treated base |
| Total pavement thickness                           | 630 mm                                                                 |
| Bituminous layer modulus                           | 3,000 MPa (VG40)                                                       |
| Tensile strain bottom of bituminous                | 127.5 / 150.0 allowable                                                |
| Vertical strain top of subgrade                    | 168.6 / 300.6 allowable                                                |
| Fatigue life / rutting life                        | 246.3 / 1,802.9 msa                                                    |
| CTB tensile strain (0.80 MPa) / life               | 44.3 / 14,400.8 msa                                                    |
| CTB cumulative fatigue damage                      | 0.788                                                                  |
| Least bituminous thickness (other layers as given) | 100 mm                                                                 |
| Result:                                            | ALL CHECKS SATISFIED                                                   |

STRAINS vs BITUMINOUS THICKNESS



LIFE vs BITUMINOUS THICKNESS



NOTES

- All dimensions in mm.
- Materials and workmanship to MoRTH Specifications (5th rev.).
- Bottom DBM: rich bottom mix, design air voids as adopted for fatigue.
- GSB extended full width with drainage and filter layers (Cl. 7.2.1).

IRC 37 example check

Flexible pavement 630 mm

Designed: - Checked: -

Date: 31 Oct 2026 Scale: as shown Sheet 1 of 1